COMMONWEALTH OF KENTUCKY NATURAL RESOURCES & ENVIRONMENTAL PROTECTION CABINET DEPARTMENT FOR ENVIRNOMENTAL PROTECTION DIVISION OF WATER

APPLICATION FOR PERMIT TO CONSTRUCT ACROSS OR ALONG A STREAM AND / OR WATER QUALITY CERTIFICATION

Chapter 151 of the Kentucky Revised Statutes requires approval from the Division of Water prior to any construction or other activity in or along a stream that could in any way obstruct flood flows or adversely impact water quality. If the project involves work in a stream, such as bank stabilization, dredging or relocation, you will also need to obtain a 401 Water Quality Certification (WQC) from the Division of Water. This completed form will be forwarded to the Water Quality Branch for WQC processing. The project may not start until all necessary approvals are received from the KDOW. For questions concerning the WQC process, contact John Dovak at 502/564-3410.

If the project will disturb more than 1 acre of soil, you will also need to complete the attached Notice of Intent for Storm Water Discharges, and return both forms to the Floodplain management Section of the KDOW. This general permit will require you to create an implement an erosion control plan for the project.

1.	OWNER: City of Owensboro; Joe Schepers, PE, SE; City Engineer 1922LA					
1.	Give name of person(s), company, governmental unit, or other owner of proposed project.					
	MAILING ADDRESS: P.O. Box 10003 / Owensboro, KY 42302-9003					
	PhONE #: 270-687-8646 EMAIL: schepersjg@owensboro.org					
2.	AGENT: R. David Weaver, PE, PLS / Bryant Engineering, Inc. Give name of person(s) submitting application, if other than owner.					
	ADDRESS: 4215 Benttree Drive					
	Owensboro, KY 42304					
	TELEPHONE #: (270) 685-2811 EMAIL: david.weaver@bryant-eng.com					
3.	ENGINEER: R. David Weaver, PE, PLS P.E. NUMBER: 19178 Contact Division of Water if waiver can be granted.					
	Contact Division of Water if waiver can be granted. TELEPHONE #: (270)685-2811 EMAIL: @bryant-eng.com					
4.	DESCRIPTION OF CONSTRUCTION: A detailed description is provided in the the project overview section. In general the project will involve ditch improvements on Harsh Ditch and retention basins. The projects has two primary goals: 1.) To lower the water surface profile so that all roadways are passable in the 25-yr event by emergency vehicles. 2.) To improve the infrastructure to allow for future combined sewer separation projects.					
5.	COUNTY: Daviess County NEAREST COMMUNITY: City of Owensboro					
6.	USGS QUAD NAME Owensboro East & Sutherland LATITUDE/LONGITUDE: N 37-44-31 / W 87-06-14					
7.	STREAM NAME: Harsh Ditch WATERSHED SIZE (in acres): 1,971					
8.	LINEAR FEET OF STREAM IMPACTED: N/A					
9.	DIRECTIONS TO SITE: The project can be accessed by taking the north bound exit ramp from the US 60 Bypass onto KY 2155 and traveling north on KY 2155 to the intersection of KY 2155 and E. 25 th . Turn left onto E 25 th Street and left onto Veach Road.					
10.	IS ANY PORTION OF THE REQUESTED PROJECT NOW COMPLETE? Yes X No If yes, identify the completed portion on the drawings you submit and indicate the date activity was completed. DATE:					
11.	ESTIMATED BEGIN CONSTRUCTION DATE: June 2009					
12.	ESTIMATED END CONSTRUCTION DATE: December 2011					
13. attach	HAS A PERMIT BEEN RECEIVED FROM THE US ARMY, CORPS of ENGINEERS? Yes X No If yes, a copy of that permit.					

4.	THE APPLICANT MUST ADDRESS PUBLIC NOTICE:					
	(a) PUBLIC NOTICE HAS BEEN GIVEN FOR THIS PROPOSAL BY THE FOLLOWING MEANS:					
	(b) I REQUEST WAIVER OF PUBLIC NOTICE BECAUSE:					
15.	Contact Division of Water for requirements. I HAVE CONTACTED THE FOLLOWING CITY OR COUNTY OFFICIALS CONCERNING THIS PROJECT:					
	City Engineer and Floodplain Administrator Give name and title of person(s) contacted and provide copy of any approval city or county may have issued.					
16.	LIST OF ATTACHMENTS: List plans, profiles, or other drawings and data submitted. Attach a copy of a 7.5 minute USGS topographic map clearly showing the project location.					
	Public Notice, Project Overview, USGS Map, Current FIRM, Pending FIRM, Construction Drawings,					
	Photographs, Hydraulic Model, & Corp Permit Application					
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7.	I, (owner) CERTIFY THAT THE OWNER OWNS OR HAS EASEMENT RIGHTS ON ALL PROPERTY					
	ON WHICH THIS PROJECT WILL BE LOCATED OR ON WHICH RELATED CONSTRUCTION WILL					
	OCCUR (for dams, this includes the area that would be impounded during the design flood).					
	REMARKS: The City of Owensboro is in the process of acquiring all of the required easements. As per phone					
	conversation on 02/18/09 with Julia Harrod, the application is being submitted in advance of all of the easements being					
	acquired.					
	I hereby request approval for construction across or along a stream as described in this application and any accompanying					
	documents. To the best of my knowledge, all the information provided is true and correct.					
	SIGNATURE: Joseph D Schears					
	DATE: 2 19/109					
	SIGNATURE OF LOCAL FLOODPLAIN COORDINATOR: Permit application will be returned to applicant if not properly endorsed by the local floodplain coordinator.					
	DATE: 3-19-09					
	SUBMIT APPLICATION AND ATTACHMENTS TO:					

Floodplain Management Section Division of Water 14 Reilly Road Frankfort, KY 40601

HARSH DITCH PHASE II DRAINAGE IMPROVEMENTS & COMBINED SEWER SEPERATION

KY DIVISION OF WATER PERMIT APPLICATION

Prepared for:
City of Owensboro
Joe Schepers, PE, SE
City Engineer
PO Box 10003
Owensboro, KY 42302-9003





Prepared by:



Brynnt engineering. Inc.

4215 BENTTREE DRIVE•P.O. BOX 21382•OWENSBORO, KY 42304 PHONE: (270)685-2811 FAX: (270)683-4991

PUBLIC NOTICE

AFFIDAVIT OF PUBLICATION

Allison Bates of Owensboro, Kentucky being first duly sworn, says that she is the Classified Supervisor of the Owensboro Messenger-Inquirer, Inc. a newspaper printed and published in the State of Kentucky, County of Daviess, and that the advertisement is a true copy which has been published in the Messenger-Inquirer on the following dates, viz: February 23rd, February 24th, February 25th, 2009

Allison Bates

Subscribed and sworn to before me, a Notary Public within and for the State and County aforesaid, by Allison Bates to me personally known, this 26th of February, 2009. My commission expires the 24th day of September 2011.

Sally E. Harris

County of Daviess
Notary Public State of Kentucky

PUBLIC NOTICE

Notice is hereby given that the City of Owensboro, P.O. Box 10003; Owensboro, KY 42302-9003, has filed an application with the Natural Resources and Environmental Protection Cabinet to make drainage improvements on Harsh Ditch from Veach Road to KY 2155. Improvements to include retention basins and channel improvements. The project is located in the City of Owensboro, beginning at the intersection of Veach Road and Harsh Ditch, 0.3 miles north of the US 60 Bypass as measured along Veach Road, and terminating at KY 2155, 1.5 miles north of the US 60 Bypass as measured along KY 2155. Any comments or objections concerning this application shall be directed to: Kentucky Division of Water, Water Resources Branch, 14 Reilly Road, Frankfort Office Park, Frankfort, KY 40601. Phone: (502)564-3410.

B6 MESSENGER-INQUIRER, Tuesday, February 24, 2009

PUBLIC NOTICE

Notice is hereby given that the City of Owensboro, P.O. Box 10003; Owensboro, KY 42302-9003, has flied an application with the Natural Resources and Environmental Protection Cabinet to make drainage improvements on Harsh Ditch from Veach Road to KY 2155. Improvements to include retention basins and channel improvements. The project is located in the City of Owensboro, beginning at the intersection of Veach Road and Harsh Ditch, 0.3 miles north of the US 60 Bypass as measured along Veach Road, and terminating at KY 2155, 1.5 miles north of the US 60 Bypass as measured along KY 2155. Any comments or objections concerning this application shall be directed to: Kentucky Division of Water, Water Resources Branch, 14 Reilly Road, Fankfort Office Park, Frankfort, KY 40601. Phone:

B6 MESSENGER-INQUIRER, Monday, February 23, 2009

S4 9

PUBLIC NOTICE

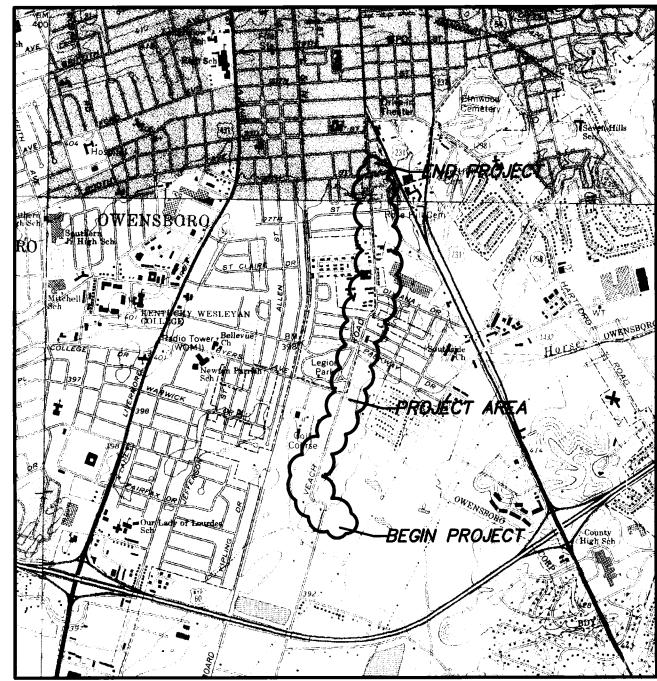
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PROJECT OVERVIEW

Project Overview

Location / Station Range	Construction Plan Sheets	Photographs #	Summary of Work
Sta. 102+00 ~ 103+00	23	1	Existing bridge to be replaced with a double 10'x6' RCBC with a 8'x6' RCBC. Existing paved ditch on the downstream side to be connected to the proposed RCBC.
Sta. 103+00 ~ 111+50	23-27	2	Existing paved ditch and berm to be removed and existing basin to be enlarged.
Sta. 111+50 ~ 113+00	27	3	Existing double 72"CMP to be replaced with a double 10'x6' RCBC with a 8'x6' RCBC. Existing paved ditches to be connected to proposed structure
Sta. 201+50 ~ 213+00	29-31	4	Existing channel to be raised for safety reasons. By raising the flowline of the existing channel the guardrail along Veach Road can be removed. In addition the existing channel has some near vertical slopes along Veach Road that will be eliminated. The existing channel is partially lined with broken concrete. A double 10'x4' RCBC will be constructed to replace the existing channel.
Sta. 213+00 ~ 218+50	33		The exsiting 54"CMP in the existing channel will be left in place. A double 10'x4' RCBC will be constructed west of the existing structure.
Sta. 218+50 ~ 303+00	33-35	5	The existing paved ditch shall be widened. The existing RCBC under Live Oak Place will remain.
Sta. 303+00 ~ 305+20	35-37		The existing CMPA to be replaced with a double 10'x4' RCBC
Sta. 305+20 ~ 406+00	37 & 53		The existing CMPA to be replaced with a paved ditch and a dry retention basin.
Sta. 406+00 ~ 418+50	37-41		The existing CMPA to be replaced with a double 10'x4' RCBC
Sta. 418+50 ~ 500+50	41-43 & 54	6	The existing dry retention basin & paved ditch to be replaced with a larger wet basin.
Sta. 500+50 ~ 503+00	43		A double 10'x4' RCBC shall be constructed to connect two proposed wet basins.
Lt Sta. 500+00 ~ 503+00	51	7&8	The old existing channel in this area shall be raised for safety reasons. The City has pulled cars out of this channel on more than one occasion.
Sta. 503+00 ~ 600+50	43-45 & 55	9	The existing channel and dry basin shall be replaced with a wet retention basin.
Sta. 600+50 ~ 601+00	45		The existing CMPA shall be replaced with a 12'x4' RCBC
Sta. 601+00 ~ 609+00	47-49	10	The existing channel shall be widened and paved.
Sta. 609+00 ~ 610+00	49		An additional RCBC barrel shall be constructed north of the existing culvert.

USGS MAPPING FOR THE PROJECT



SCALE: 1"=2000'

PREPARED BY:

BRYANT ENGINEERING INC.

4215 BENTTREE DRIVE P.O. BOX 21382 OWENSBORO, KENTUCKY 42304

DATE: 2-17-2009

HARSH DITCH (PROJECT AREA)

CITY OF OWENSBORO DAVIESS COUNTY, KENTUCKY

> OWENSBORO EAST AND

SUTHERLAND QUADRANGLE

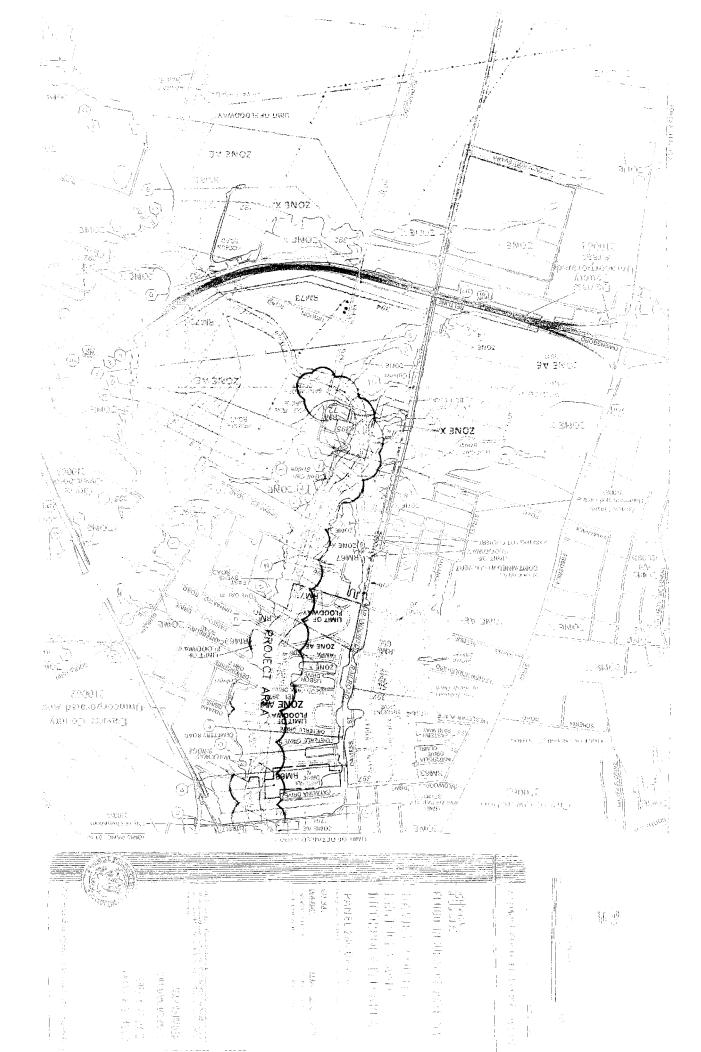
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LONGITUDE: W 87° 06' 21"

END: LATITUDE: N 37° 45' 10" LONGITUDE: W 87° 06' 08"

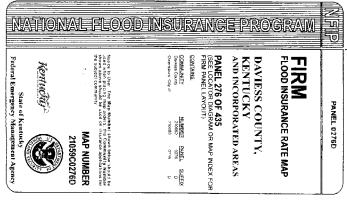
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CURRENT FIRM MAPPING FOR THE PROJECT



PENDING FIRM MAPPING FOR THE PROJECT







FIRM
FLOOD INSURANCE RATE MAP
FLOOD INSURANCE RATE MAP
FLOOD INSURANCE RATE MAP
AND INCORPORATED AREAS
AND INCORPORATED AREAS
AND INCORPORATED AREAS
SELECTOR OF 435
SEE LOCATOR OF GORRAM OR MAP INDEX FOR
FRIM MAPAGE LAYOUT
COMMAND

REDUCED CONSTRUCTION PLANS

CITY OF OWENSBORO

THE FOLLOWING DRAWINGS SHALL BE INCLUDED BY REFERENCE

KYTC STANDARD DRAWINGS

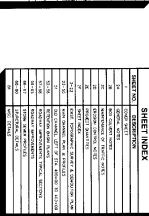
EXHIBIT NO. X-XX OMPC PUBLIC IMPROVEMENT SPECIFICATIONS STANDARD DRAWINGS

HARSH DITCH PHASE II

KIA LOAN NO.: OWENSBORO BID NO.:

STA. 100+00 - 113+00

SET NO.:



VICINITY MAP

REDUCED PLAN SET DO NOT SCALE

COMMISSIONERS
WAYOR PRO TEM
CHARLIE CASTLEN JOHN M. KAZLAUSKAS DAVID JOHNSON CANDANCE BRAKE MAYOR

> PROJECT:_ CITY OF OWENSBORO KENTUCKY DAVIESS HARSH DITCH DRAINAGE IMPROVEMENTS PHASE II

CITY OF OWENSBORO

ENGINEERING

COVER SHEET

CITY OF OWENSBORO

OWNER/DEVELOPER

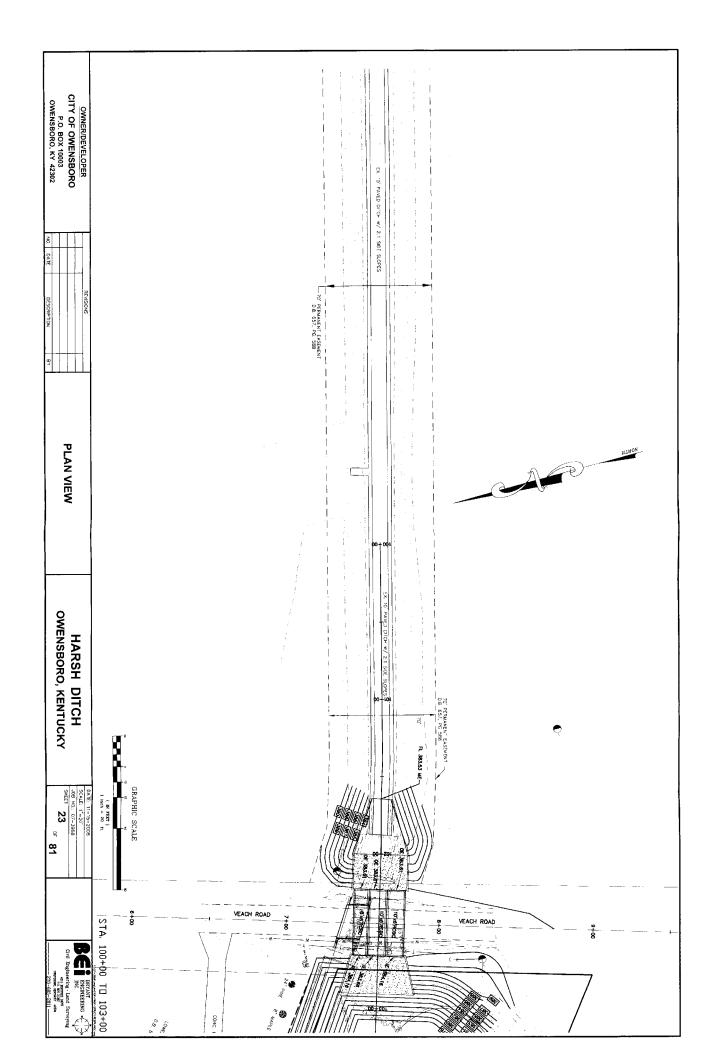
Daviess County

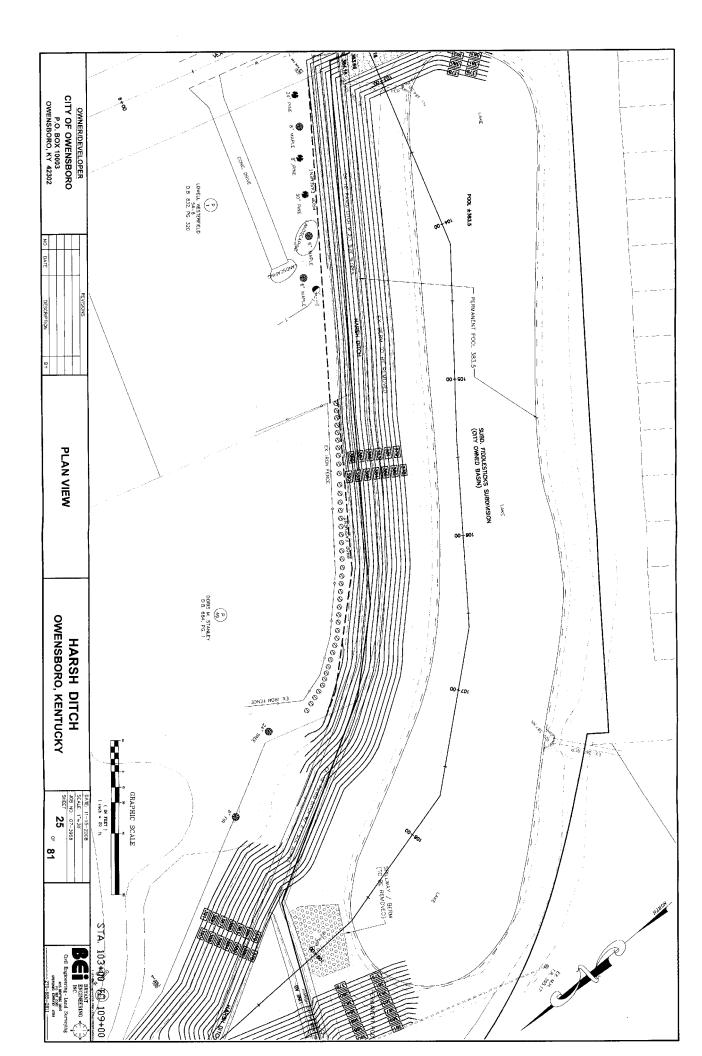
アペン **K**entucky

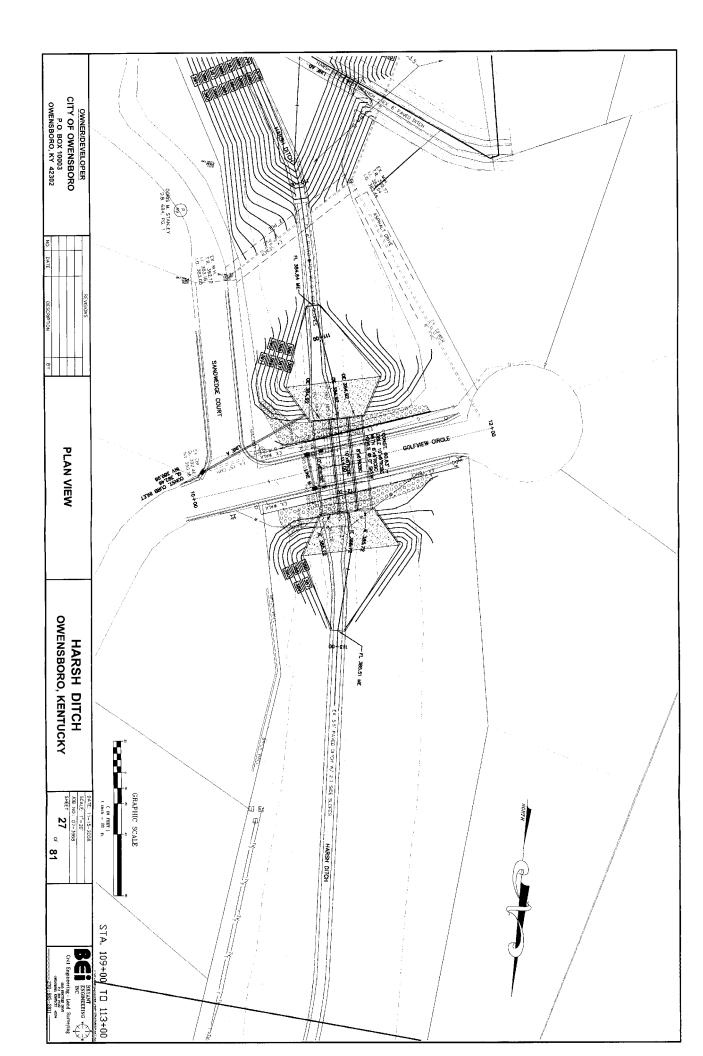
P.O. BOX 10003 OWENSBORO, KY 42302

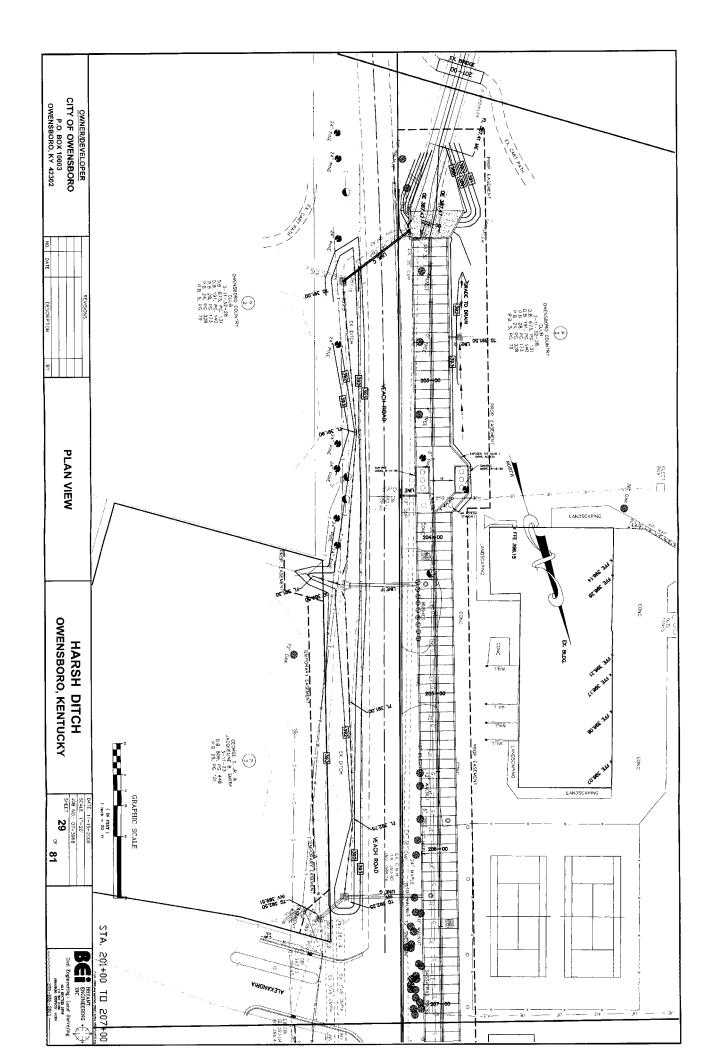
OWENSBORO, KENTUCKY HARSH DITCH

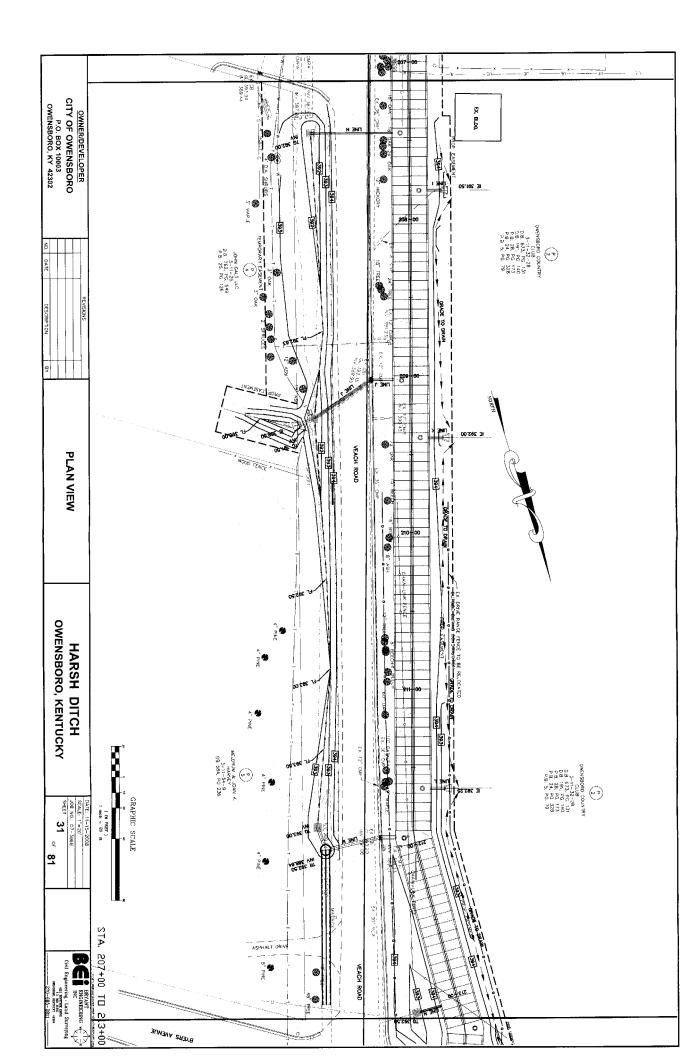
SCALE: N.T.S JOB NO.: 07-3958 SHEET ့ **8**

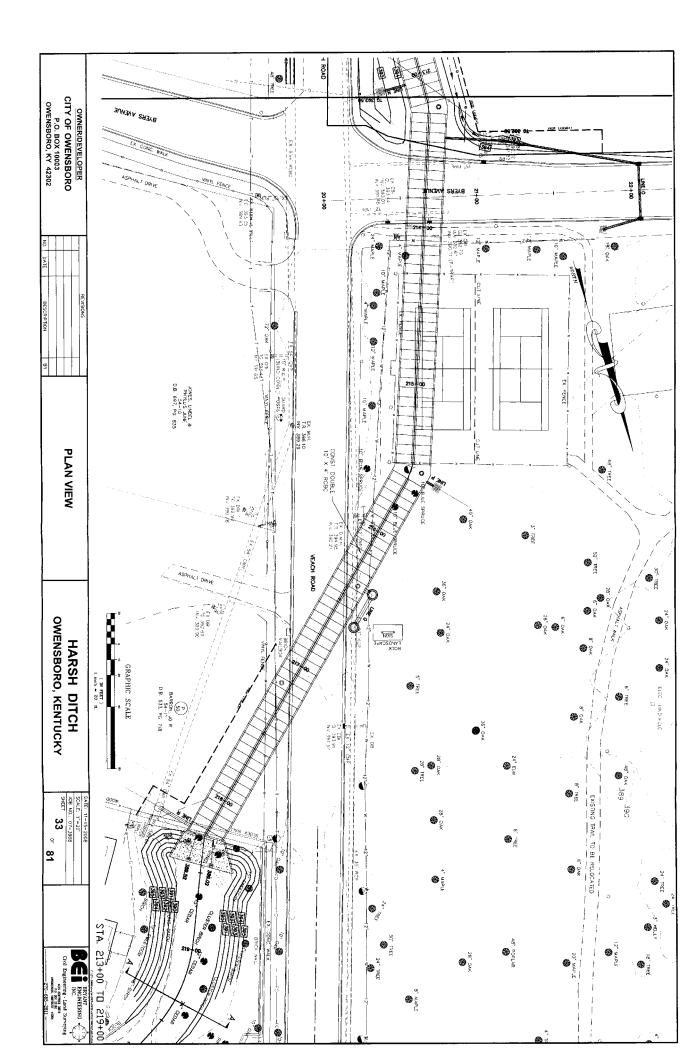


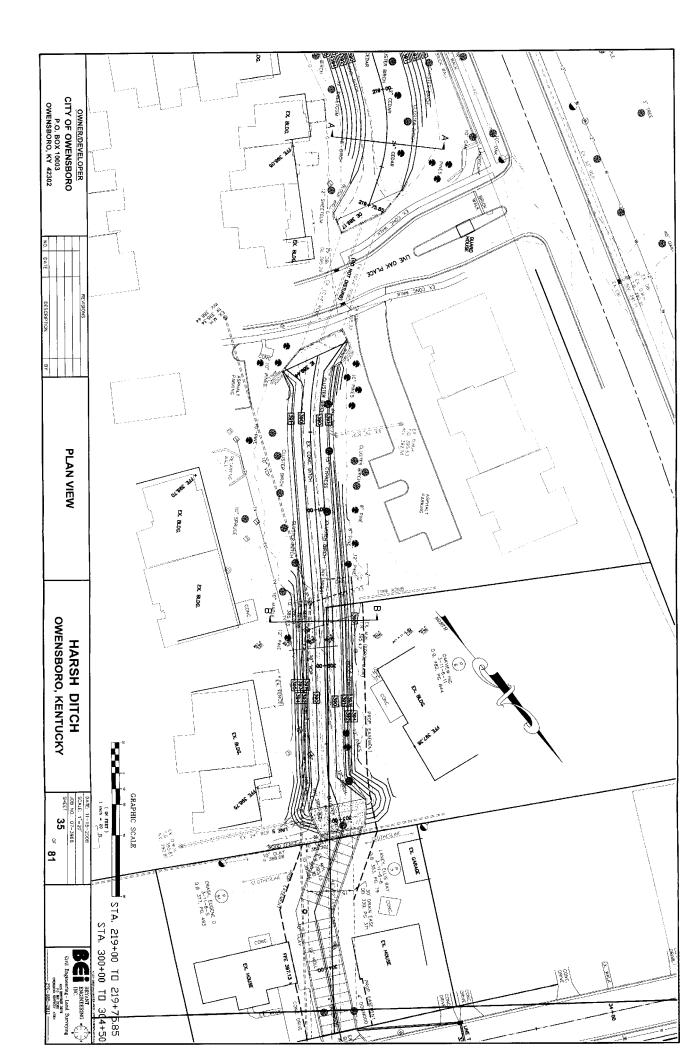


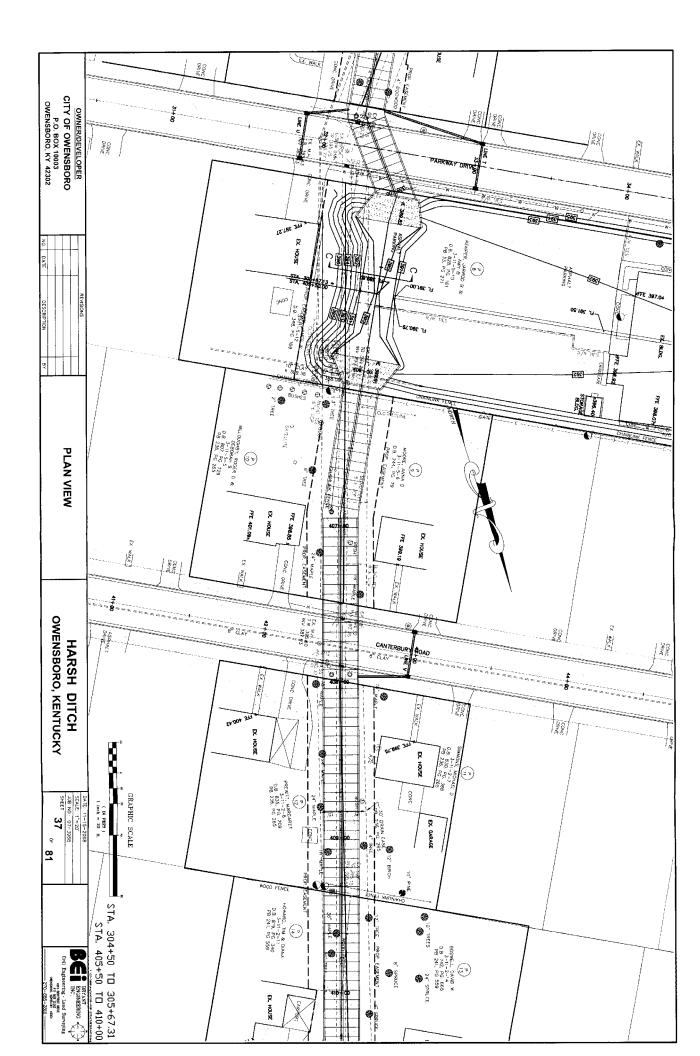


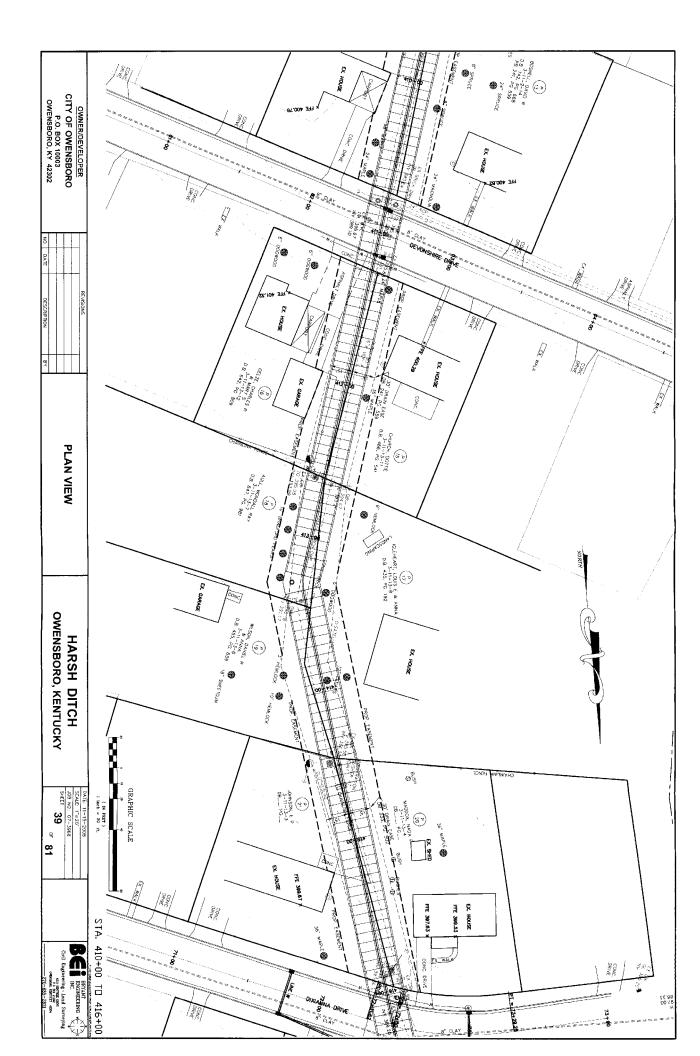


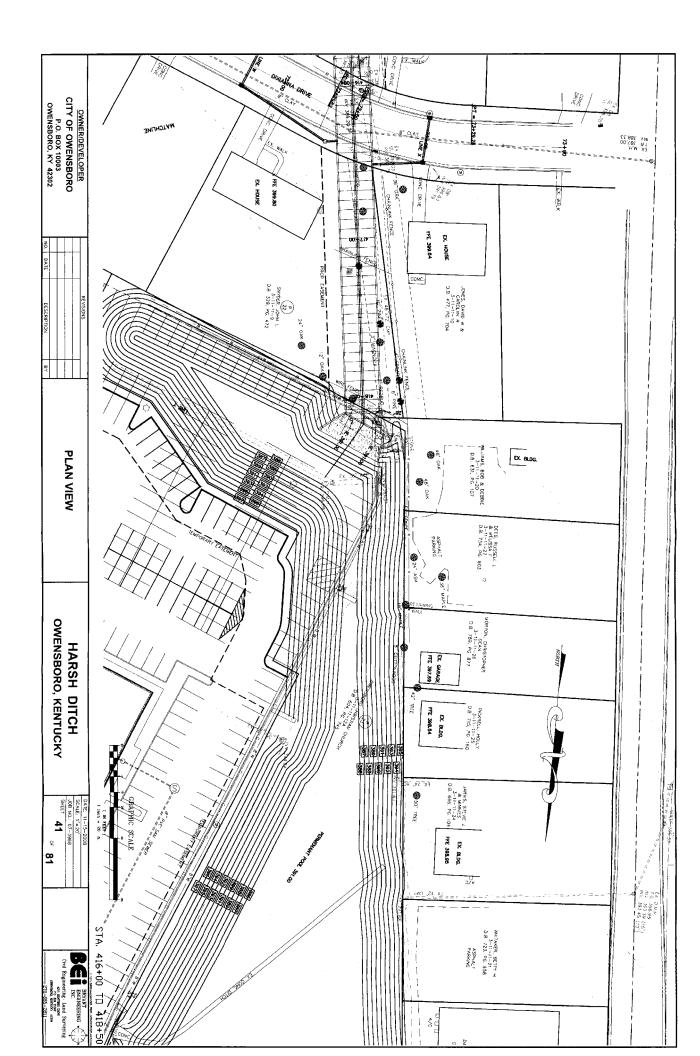


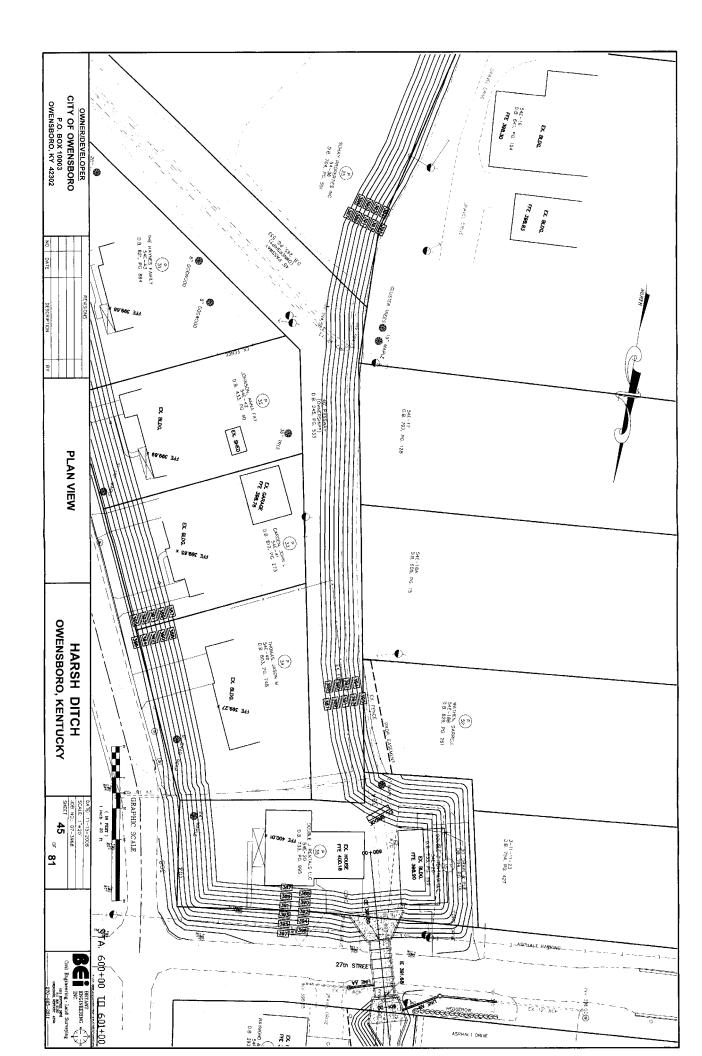


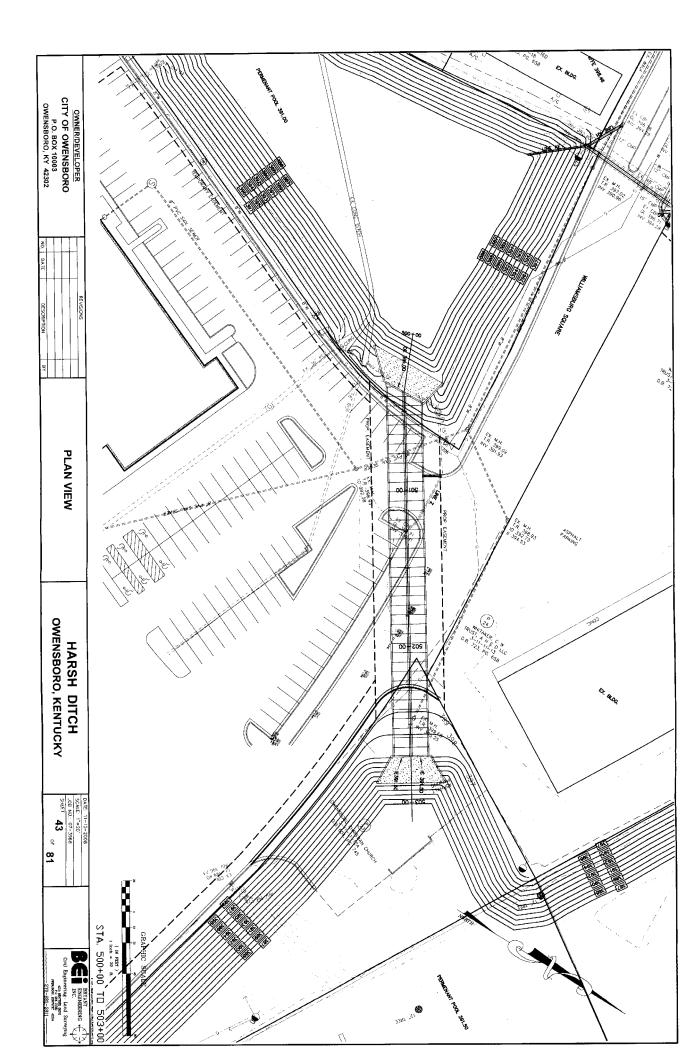


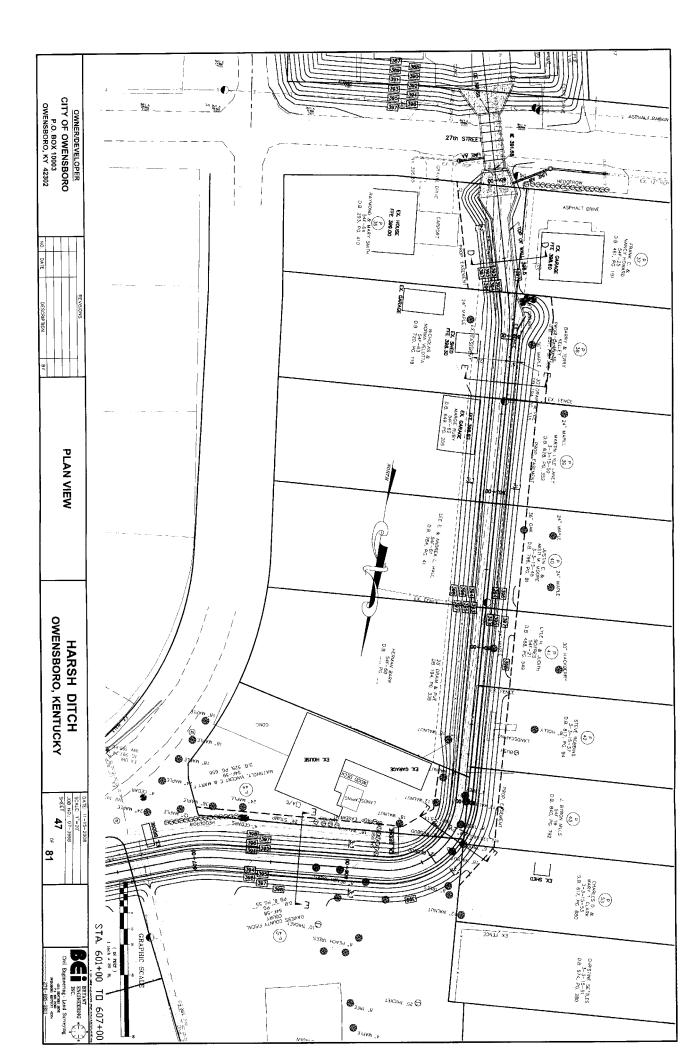


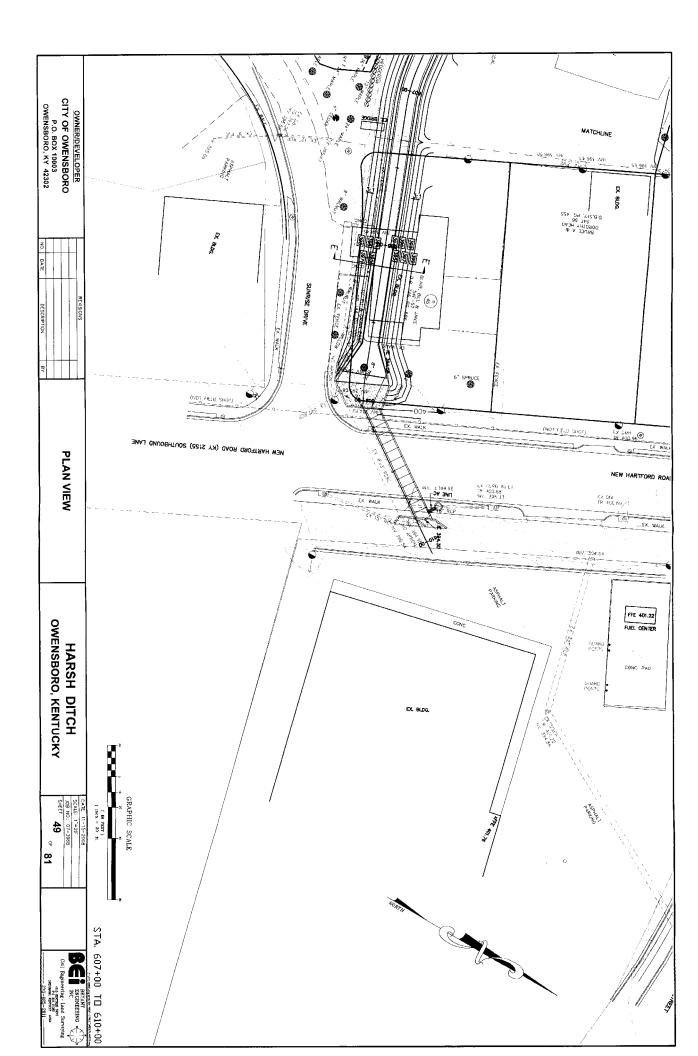


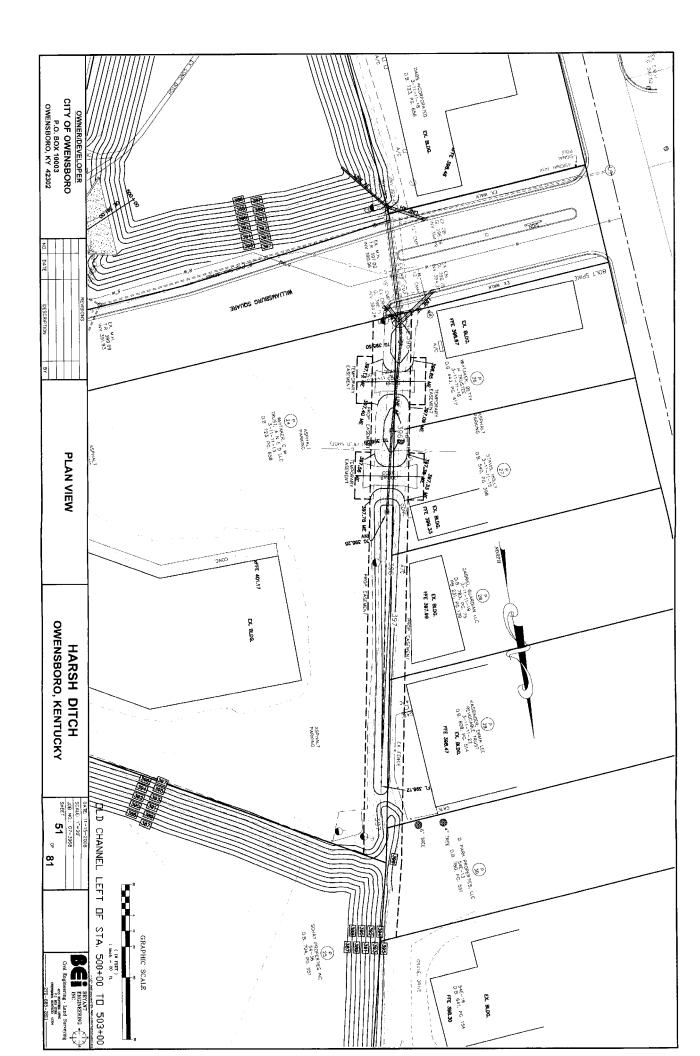


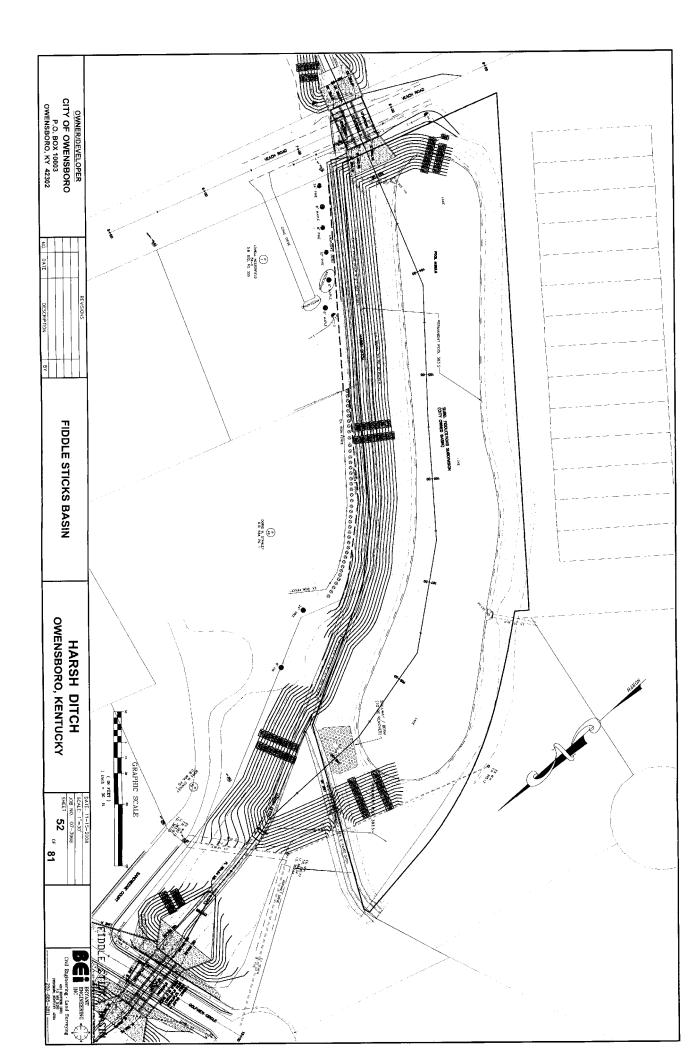


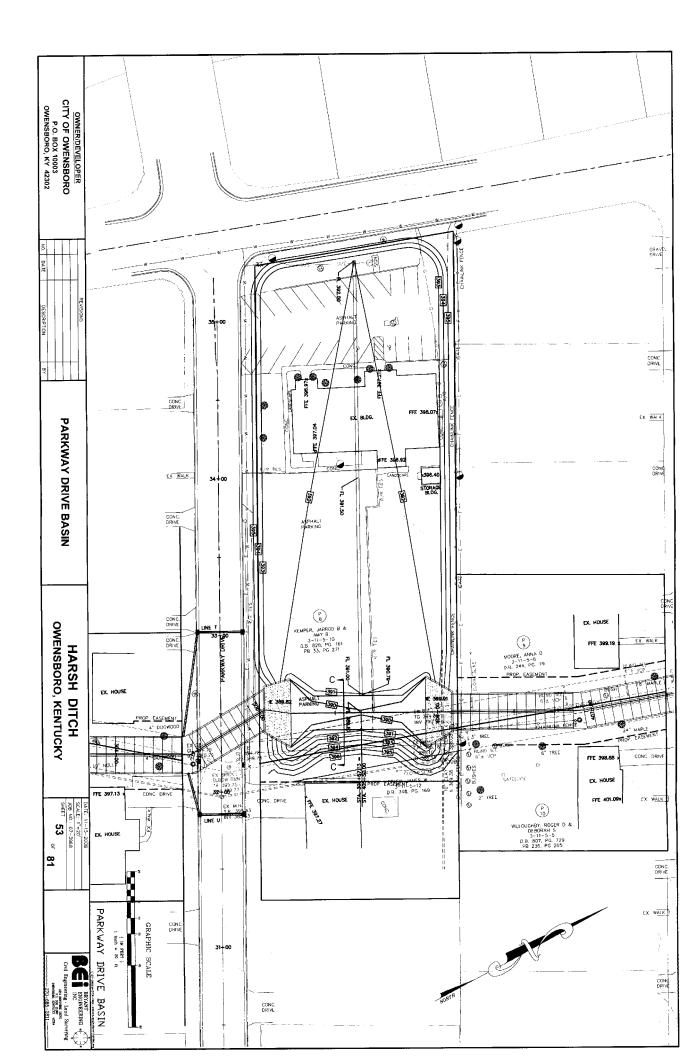


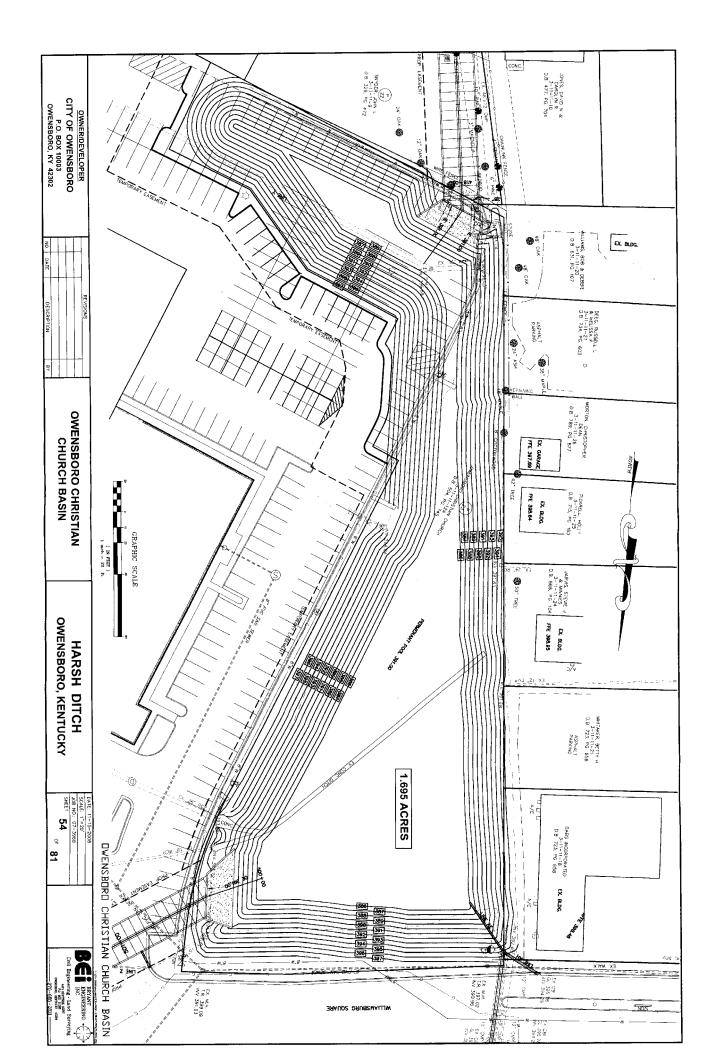


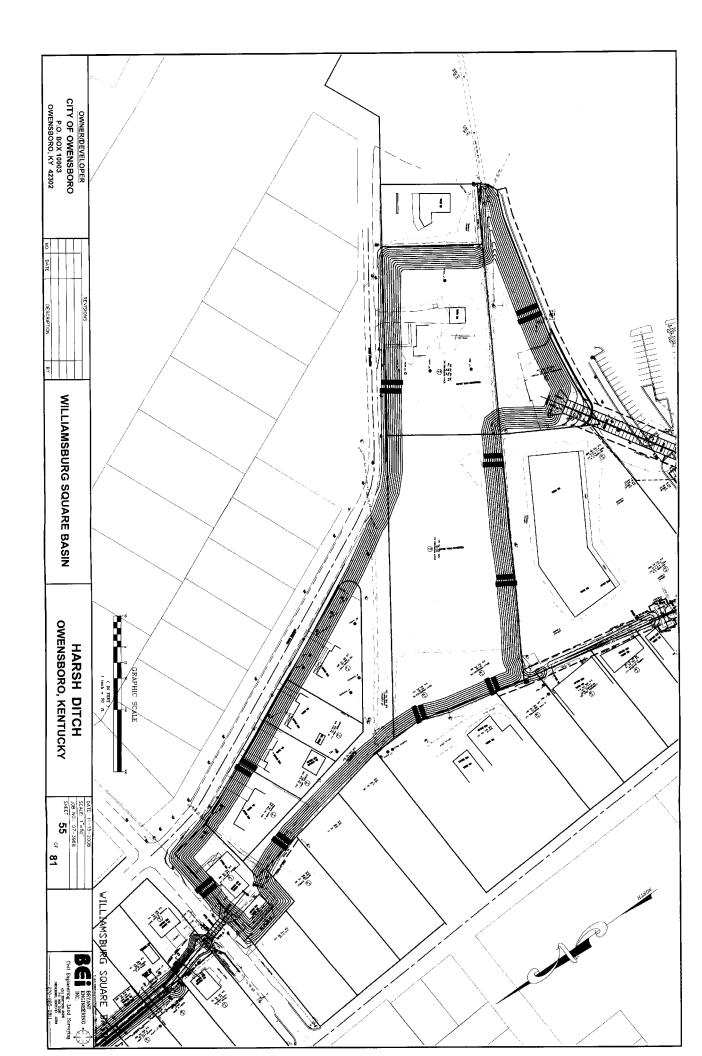


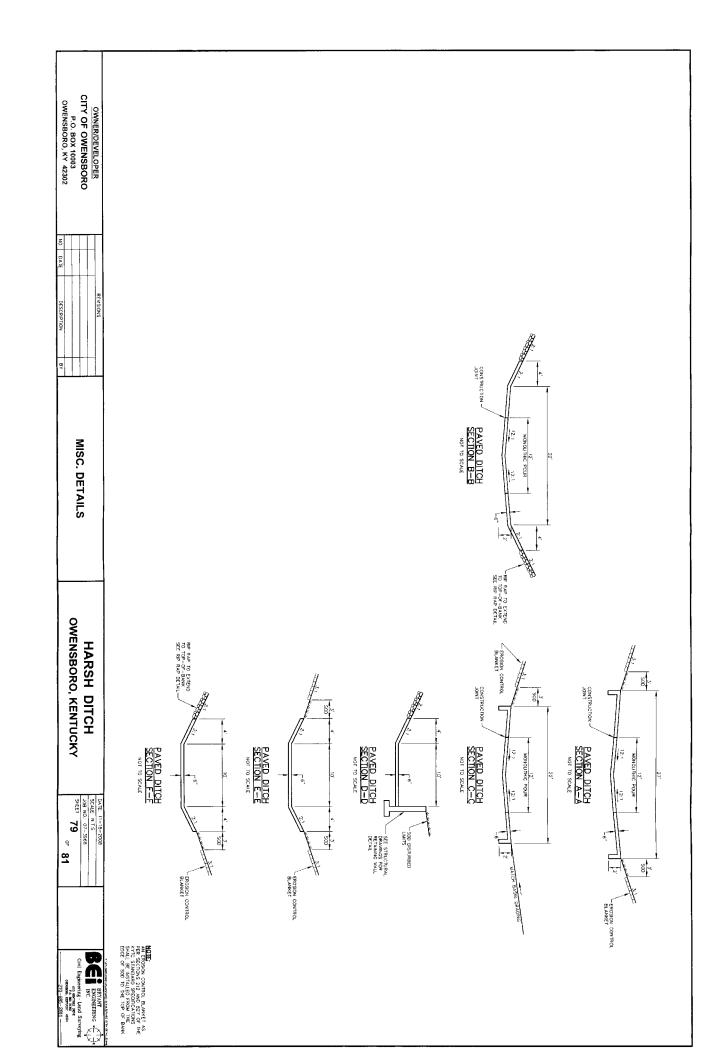












PROJECT PHOTOGRAPHS

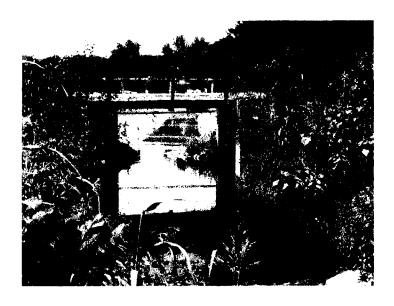


Photo.#1
Existing bridge to be replaced with box culvert



<u>Photo. #2</u>

Existing berm & paved ditch to be removed. The existing basin to be enlarged.

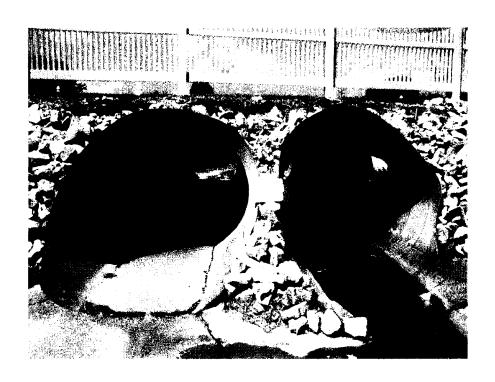


Photo. #3

Existing double 72" CMP to be removed and replaced with double 10'x4' RCBC

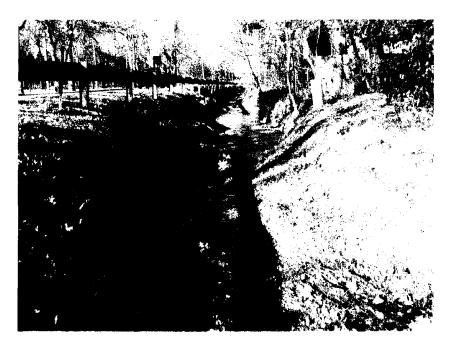


Photo. #4Existing channel to be raised and guardrail eliminated



Photo. #5Existing concrete channel to be widened



<u>Photo. #6</u>

Existing dry basin to be replaced with a larger wet basin.

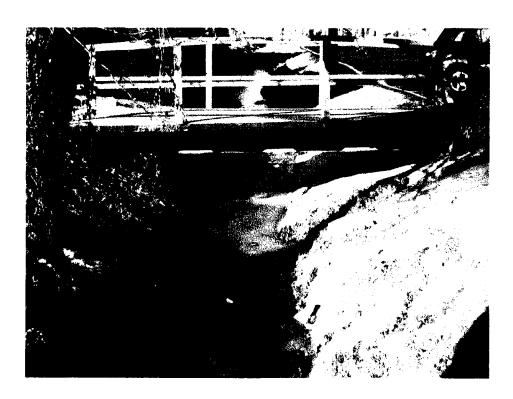


Photo. #7Existing channel to be raised.



Photo. #8

Existing channel to be raised. Picture taken during a high water event.



Photo. #9

Existing dry basin during high water event. Basin to be enlarged as a wet basin.



Photo. #10

Existing channel to be enlarged and paved.

HYDRAULIC MODEL

HYDROLOGIC AND HYDRAULIC ANALYSIS FOR HARSH DITCH

CITY OF OWENSBORO, KENTUCKY

FINAL REPORT

FEBRUARY 2009

Prepared for:

Bryant Engineering, Inc.
P.O. Box 21382
Owensboro, KY 42304

Prepared by:

CDP Engineers 3250 Blazer Parkway Lexington, KY 40509

HYDROLOGIC AND HYDRAULIC ANALYSIS

Harsh Ditch Watershed

Harsh Ditch comprises an approximately 1,971-acre drainage area extending from just south of the U.S. HWY 60 Bypass at its confluence with Horse Fork and is bounded by Frederica Street to the west and by Parrish Avenue to the east. Currently, Harsh Ditch does not drain the entire watershed due to a combination sewer system that accepts both sanitary and storm water flows. Within the Harsh Ditch watershed there are 732-acres of combination sewer areas and 1,239-acres of straight storm water areas. Harsh ditch as it is designed today (2009) only drains that portion of the watershed which contains the straight storm-water areas.

Harsh Ditch is a backwater controlled system. During rain events the backwater originates in Horse Fork and the elevated water surface propagates upstream into Harsh Ditch. In addition to the backwater from Horse Fork the Main Channel of Harsh Ditch also exhibits drainage problems created by lengthy stretches of open channel having been replaced by undersized drainage pipe laid on a flat grade. Other flooding problems along the Main Channel are due to the naturally flat topography and areas located in depressions which cannot develop sufficient head to drain until backwater elevations downstream have abated.

For the purpose of this study three (3) models were developed using the Hydrologic Engineering Center's Hydrologic Modeling System (HEC-HMS) version 3.1.0 for the hydrologic analysis and the Hydrologic Engineering Center's River Analysis System (HEC-RAS) version 4.0 for the hydraulic analysis. The models presented in this report are: Model #1 which is a calibration model to the 1997 FIRM, Model #2 is an existing conditions model for Harsh Ditch as it is in 2009, Model #3 shows improvements proposed for Harsh Ditch to reduce flooding in the streets for the current drainage area of 1,239-acres with an additional 15-acres of combination sewer area.

Project Goals

The goal for the Harsh Ditch Phase II Drainage Improvement project as set forth by the City of Owensboro is to reduce flooding in the streets so that emergency vehicles can access citizens' homes for the 25yr event and the additional goal of storm-water removal from the combination sewer system. This project also seeks to provide a main storm-water conveyance system that can be added to in the future. Improvements proposed for Harsh Ditch follows the Owensboro storm-water master plan which recommends construction of retention basins and channel reconstruction on portions of the main Harsh Ditch branch with a series of box culverts and paved channels. These improvements will add more storage capacity to the undersized system and will allow a small portion of storm-water to be removed from the combination sewer system.

Hydrologic Methodology

Hydrological analysis was carried out to establish the peak discharge-frequency relationship for Harsh Ditch. The hydrologic computations were performed with the computer program HEC-HMS version 3.1.0. Hydrologic parameters were based on the Soil Conservation Service (SCS) Curve Number (CN) Methodology outlined in <u>Urban Hydrology</u> for Small Watersheds, Technical Release 55 (TR-55).

Design Storm

The SCS Type II distribution 24-hr storm duration was used to simulate the temporal distribution of rainfall in three minute intervals. The total volume of rain for each return frequency was obtained from Kentucky *Engineering Memorandum No. 2: Rainfall Frequency Values for Kentucky, Revised June 1, 1979* and is shown in Table 1.

Table 1 Rainfall Depths for Selected Storm Frequencies

Storm Duration (hour)	Storm Frequency (year)	Rainfall Depth (inches)
24	10	4.7
24	25	5.5
24	50	6.0
24	100	6.5

Drainage Area

The watershed and sub-watersheds were delineated using a previous study performed by Johnson, Depp and Quisenberry Consulting Engineers and basemap layers (two-foot contour mapping and aerial photograph) supplied by the City of Owensboro. Harsh Ditch watershed is approximately 1,971-acres, 732-acres are drained by a combination sewer system and the remaining 1,239-acres of dedicated storm water drains into Harsh Ditch. The watershed was divided into sub-watersheds approximately ranging in size from 0.02 – 1.03 square miles for three (3) different models. The different scenarios are listed below:

Model #1 (Calibration Model):

Total Area = 1,971-acres (100% combination sewer areas) Table 2

Model #2 (Existing Conditions):

Total Area = 1,239-acres (0% combination sewer areas) Table 3

Model #3 (Proposed Improvements):

Total Area = 1,254-acres (2% combination sewer areas) Table 4

Time of Concentration

Time of concentration values were calculated for each sub-watershed using the methodology outlined in TR-55. Times of concentration for this study ranged from 13 to 75 minutes.

Curve Number Generation

A weighted curve number was generated for each sub-watershed using the SCS Hydrologic Soil Group (HSG) and land use coverages as outlined in TR-55.

Table 2
Sub-Watersheds With 100% of Combined Sewer Areas

SUB WATERSHED	100% CSO (SQMI)	100% CSO (Acres)	Time of Concentration (min)	CN	10-year (cfs)	25-year (cfs)	50-year (cfs)	100-year (cfs)
1	0.27	175.3	52	74.59	158.7	208.4	240.4	272.9
2	0.55	352.3	73	85.02	366.4	453.4	508.3	563.2
3	0.06	40.6	26	74.79	57.9	75.9	87.5	99.2
4	1.03	661.6	75	79.05	549.7	702.6	800	898.2
5	0.04	25.7	36	79.75	37.4	47.6	54	60.5
6	0.16	99.7	28	82.11	193.1	242.1	273	304.1
7	0.06	36.5	13	84.53	122.7	151.4	169.5	187.6
8	0.08	52.4	13	78.75	136	173.2	196.8	220.6
81	0.21	131.7	33	78	195.7	251.2	286.6	322.3
82	0.02	12.5	14	78	31.7	40.6	46.3	52
9	0.02	9.6	17	81.22	32	40.2	45.5	50.7
10	0.06	40	14	82.95	112.4	140.1	157.5	175
11	0.03	20.2	17	81.28	48	60.4	68.3	76.2
12	0.49	312.4	60	79.61	313.2	398.9	453.5	508.6
Totals	3.08	1971	<u> </u>					

Table 3
Sub-Watersheds With 0% of Combined Sewer Areas

SUB WATERSHED	0% CSO Existing (SQMI)	0% CSO Existing (Acres)	Time of Concentration (min)	CN	10-year (cfs)	25-year (cfs)	50-year (cfs)	100-year (cfs)
1	0.27	173.2	52	74.59	158.7	208.4	240.4	272.9
2	0.55	354.4	73	85.02	366.4	453.4	508.3	563.2
3	0.06	40.6	26	74.79	57.9	75.9	87.5	99.2
4	0.38	244.5	50	79.05	274.1	350.1	398.3	447.1
5	0.03	19.8	36	79.75	28.1	35.7	40.5	45.4
6	0.16	99.7	28	82.11	193.1	242.1	273.0	304.1
7	0.06	36.4	13	84.53	122.7	151.4	169.5	187.6
8	0.08	52.5	13	78.75	136.0	173.2	196.8	220.6
81	0.21	131.7	33	78	195.7	251.2	286.6	322.3
82	0.02	12.5	14	78	31.7	40.6	46.3	52.0
9	0.01	6.2	17	81.22	32.0	40.2	45.5	50.7
10	0.04	28.2	14	82.95	112.4	140.1	157.5	175.0
11	0.04	22.7	17	81.28	48.0	60.4	68.3	76.2
12	0.03	16.5	15	79.61	16.2	20.5	23.2	26.0
Totals	1.94	1239						

Table 4
Sub-Watersheds With 2% of Combined Sewer Areas

SUB WATERSHED	2% CSO FUTURE (SQMI)	2% CSO FUTURE (Acres)	Time of Concentration (min)	CN	10-year (cfs)	25-year (cfs)	50-year (cfs)	100-year (cfs)
1	0.27	170.20	52	74.59	158.7	208.4	240.4	272.9
2	0.56	357.45	73	85.02	373.0	461.7	517.5	573.5
3	0.06	40.65	26	74.79	57.9	75.9	87.5	99.2
4	0.38	246.00	50	79.05	274.1	350.1	398.3	447.1
5	0.04	25.66	36	79.75	37.4	47.6	54.0	60.5
6	0.13	80.54	28	82.11	156.9	196.7	221.8	247.1
61	0.03	19.17	10	82.11	64.2	80.2	90.2	100.3
7	0.06	36.42	13	84.53	122.7	151.4	169.5	187.6
8	0.08	54.00	13	78.75	136.0	173.2	196.8	220.6
81	0.21	131.72	33	78	195.7	251.2	286.6	322.3
82	0.02	12.55	14	78	31.7	40.6	46.3	52.0
9	0.02	15.50	17	81.22	32.0	40.2	45.5	50.7
10	0.06	40.03	14	82.95	112.4	140.1	157.5	175.0
11	0.03	20.22	17	81.28	48.0	60.4	68.3	76.2
12	0.01	3.94	15	79.61	16.2	20.5	23.2	26.0
12B	0.27	170.20	52	74.59	158.7	208.4	240.4	272.9
Totals	1.96	1254						

Hydraulic Methodology

The hydraulic computations were performed with the HEC-RAS version 4.0 computer program. Input parameters used in the model were based on the infrastructure information collected by Bryant Engineering Inc. In addition the City of Owensboro supplied GIS data (two-foot contours, ditch and pipe locations, transportation features and parcel delineation) which were used to augment the survey data. Manning's roughness values for the storm sewer and open-channels were based on the values published in *Storm Water Collection Systems Design Handbook, 2001*. Modeling of Harsh Ditch includes the 10-, 25-, 50- and 100-year annual chance events based on peak discharges calculated in the hydrologic analyses. Modeling assumed that storm water collection system was free of debris and in good condition.

Calibration Model

The calibration model is essentially a FEMA Corrected Effective Model that incorporates updated hydraulic structure information and cross sections created with more detailed topographic information. The calibration model duplicates the water surface elevations shown in the 1997 FIS profiles and in the Floodway Data Table to within 0.5ft as required by FEMA (in most locations).

Table 5 shows the water surface elevations generated from the 2008 calibration model compared to the water surface elevations shown in the 1997 FIS. It can be seen that the 2008 calibration model's water surface elevations deviate from the 1997 FIS elevations by more than 0.5ft at cross sections P, Q, R and S. These differences can be attributed to improved geometry data used in the 2008 model. The 1997 FIS model used interpolated and repeated cross sections that are not representative of this portion of Harsh Ditch. Cross sections P, Q, R and S are compared to the cross sections used in the 2008 model in Figures 1-4. It can be seen that the cross sections used in the 2008 model have higher bank stations and are generally more constricted than the 1997 FIS cross sections.

To show that the 2008 surveyed cross sections are more accurate, the 1997 FIS cross sections were used in the 2008 model at locations P, Q, R and S. The results from this model can be seen in Table 6 which shows that the water surface elevations duplicate the 1997 FIS elevations. It can be further shown in photos taken from a 2-day 4.18 rain event (almost 10-yr event) which occurred on March 19, 2008 that the double 48" CMP at Williamsburg square was overtopped. This structure is located in the vicinity of cross section Q where the 1997 FIS publishes a 100-yr elevation of 397.5. The crown of the road at this location is approximately 397. Therefore, it appears that a less than 10-yr event is within 0.5ft of the 100-yr water surface elevation. Due to this photographic documentation and that repeated and interpolated cross sections were used in these locations for the 1997 FIS, the engineer believes that the recently surveyed cross sections used in the 2008 model are more accurate.

Table 5
Calibration Model with New Surveyed Cross Sections

Cross Section	1997 FIS Regulatory ¹ (FEET NGVD)	2008 Model Regulatory ¹ (FEET NGVD)	Diff. (FEET)	1997 FIS Without Floodway ² (FEET NGVD)	2008 Model Without Floodway ² (FEET NGVD)	Diff. (FEET)
Harsh Ditch						
В	394.4	394.4	0.0	390.2	390.2	0.0
c	394.4	394.5	0.1	392.4	392.8	0.4
D	394.4	394.5	0.1	392.4	392.8	0.4
E	394.5	394.5	0.0	393.4	393.2	0.2
F	394.9	394.9	0.0	394.7	394.7	0.0
G	394.9	394.9	0.0	394.7	394.7	0.0
н	394.9	394.9	0.0	394.7	394.7	0.0
i i	394.9	394.9	0.0	394.7	394.7	0.0
J	394.9	394.9	0.0	394.7	394.7	0.0
K	394.9	394.9	0.0	394.7	394.7	0.0
L	394.9	394.9	0.0	394.8	394.9	0.1
М	395.8	395.8	0.0	395.8	395.8	0.0
N	396.4	396.1	0.3	396.4	396.2	0.2
0	396.4	396.5	0.1	396.4	396.5	0.1
P	397.3	398.1	0.8*	397.3	398.1	0.8*
Q	397.5	398.3	0.8*	397.5	398.3	0.8*
R	397.9	399.2	1.3*	397.9	399.2	1.3*
S	398	399.8	1.8*	398	399.8	1.8*

¹With Backwater

²Without Backwater

^{*}Locations with water surface increase greater than 0.5ft

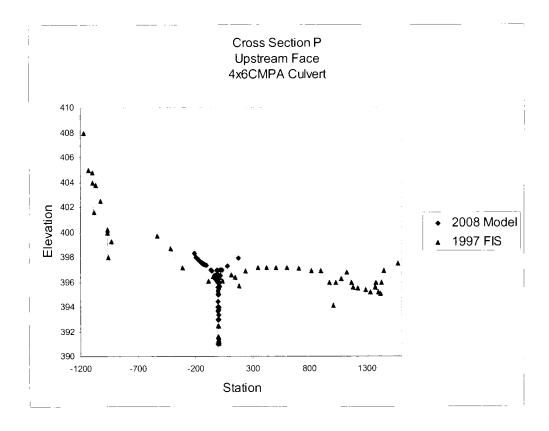


Figure 1

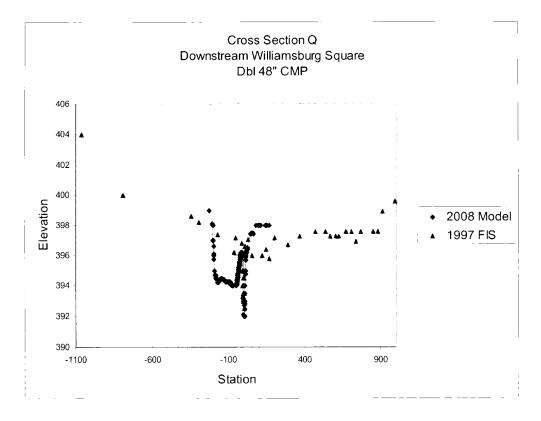


Figure 2

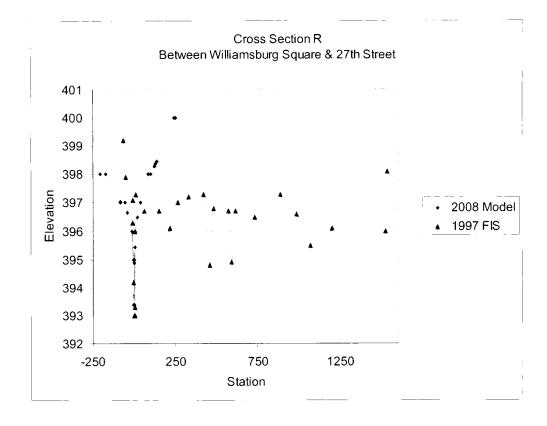


Figure 3

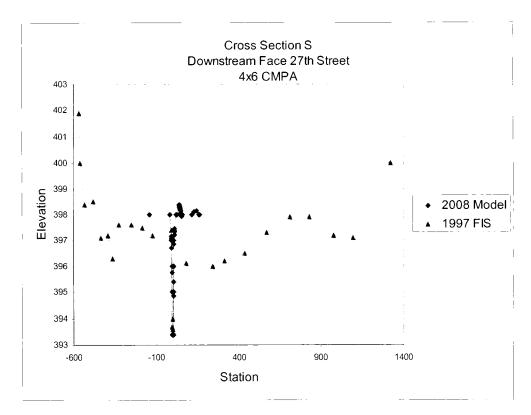


Figure 4

Table 6
Calibration Model with New Surveyed Cross Sections & Cross Sections from the 1997 FIS (Owensboro Christian Church to 27th Street)

Cross Section	1997 FIS Regulatory ¹ (FEET NGVD)	2008 Model w/FEMA XS Regulatory ¹ (FEET NGVD)	Diff. (FEET)	1997 FIS Without Floodway ² (FEET NGVD)	2008 Model w/FEMA XS Without Floodway ² (FEET NGVD)	Diff. (FEET)
Harsh						
Ditch						
В	394.4	394.5	0.1	390.2	390.2	0.0
С	394.4	394.5	0.1	392.4	392.8	0.4
D	394.4	394.5	0.1	392.4	392.8	0.4
E	394.5	394.5	0.0	393.4	393.2	0.2
F	394.9	394.9	0.0	394.7	394.7	0.0
G	394.9	394.9	0.0	394.7	394.7	0.0
H	394.9	394.9	0.0	394.7	394.7	0.0
ı	394.9	394.9	0.0	394.7	394.7	0.0
J	394.9	394.9	0.0	394.7	394.7	0.0
K	394.9	394.9	0.0	394.7	394.7	0.0
L	394.9	394.9	0.0	394.8	394.9	0.1
М	395.8	395.8	0.0	395.8	395.8	0.0
N	396.4	396.2	0.2	396.4	396.2	0.2
0	396.4	396.5	0.1	396.4	396.5	0.1
P*	397.3	397.5	0.2	397.3	397.5	0.2
Q*	397.5	397.9	0.4	397.5	397.9	0.4
R*	397.9	398	0.1	397.9	398	0.1
S*	398	398	0.0	398	398	0.0

¹With Backwater

²Without Backwater

^{*}Locations with imported cross sections from the 1997 FIS

Existing Conditions Model

The existing conditions model is representative of how Harsh Ditch is today without improvements and is used in this study as a base from which to judge proposed upgrades. The existing structures are shown in Table 7. Overall, the structures are well undersized and create high backwater during large storms. Figure 5 shows the profile for the existing conditions model which currently accepts 0% of the combination sewer area.

Table 7 Existing Structures

Location	Quantity	Туре	Length (ft)
Parking Lot Entrance	Single	3x4 CMPA	56
East 25th Street	Single	42" RCP	323
New Hartford Road	Single	3x8 RCBC	80
Residential Garage	Single	40" CMP	86
East 27th Street	Single	4x6 CMPA	27
Ridgeway Drive	Double	48" CMP	72
Devonshire	Single	4x6 CMPA	1495
Live Oak Place	Double	4x10 RCBC	73
East Byers Avenue	Single	54" RCP	580
Alexandria Place	Double	3x5 CMPA	80
Veach Road #2	Single	54" CMP	60
Owensboro CC Cart Path	Single	4x11.5 CMPA	34
Golfview Circle	Double	72" CMP	52
Veach Rd #1	Single	8x13 RCBC	25

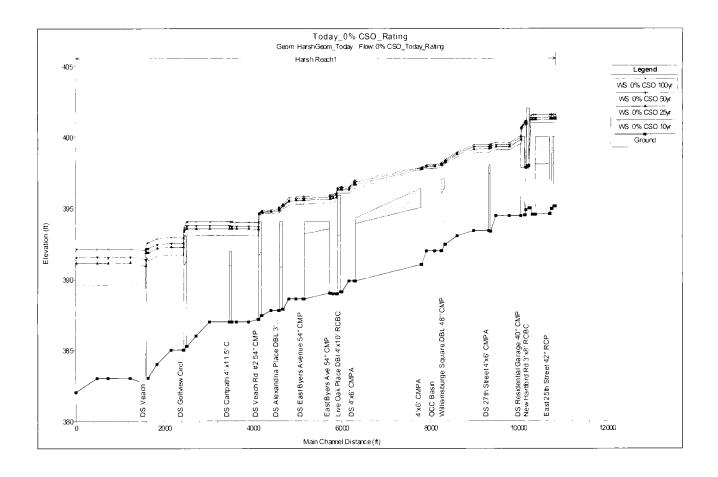


Figure 5 Existing Conditions Profile (0% CSA)

Proposed Improvements Model

Due to the low runoff velocities ubiquitous throughout the watershed it was necessary to increase the capacity of Harsh Ditch to convey the flow. The proposed improvements model is representative of how Harsh Ditch will function with increases in channel size, drainage structure size and construction of inline retention areas. The proposed structure improvements are shown in Table 8 and the proposed retention basins are shown in Table 9. Figures 6 and 7 show the water surface profiles for the proposed improvements model which will accept an additional 2% or fifteen (15) acres of the combination sewer area with and without backwater conditions respectively.

Table 8
Proposed Structure Improvements

	EXISITNG			PROPOSED		
Location	Quantity	Туре	Length (ft)	Quantity	Type	Length (ft)
Parking Lot Entrance	Single	4x3 CMPA	56		TO REMAIN	
East 25th Street	Single	42" RCP	323		TO REMAIN	
New Hartford Road	Single	8x3 RCBC	80	Double	8x3 RCBC	80
Residential Garage	Single	40" CMP	86	A SAME OF THE PARTY OF THE PART	O BE REMOVED	
East 27th Street	Single	6x4 CMPA	27	Single	12x6 RCBC	36
Ridgeway Drive	Double	48" CMP	72	Double	10x4RCBC	233
Canterbury Rd.	Single	6x4 CMPA	1495	Double	10x4RCBC	1209
Parkway Drive	CURRE	NTLY OPEN CHA	ANNEL	Double	10x4RCBC	197
Live Oak Place	Double	10x4 RCBC	73		TO REMAIN	
East Byers Avenue	Single	54" CMP	580			
Alexandria Place	Double	5x3 CMPA	80	Double	4x10 RCBC	1618
Veach Road #2	Single	54" CMP	60			
Owensboro CC Cart Path	Single	4x11.5 CMPA	34		TO REMAIN	
Golfview Circle		70" CMD	50	Double	10x6 RCBC	60
·	Double	72" CMP	52	Single	8x6 RCBC	60
Veach Rd #1	Single	12v0 DCDC	25	Double	10x6 RCBC	43
	Single	13x8 RCBC	25	Single	8x6 RCBC	43

Table 9
Proposed Retention Basins

Description	Retention Volume (ac-ft)
Fiddlesticks Basin	18.05
Parkway Drive Basin	2.69
Owensboro Christian Church Basin	8.22
North Williamsburg Square Basin	33.83

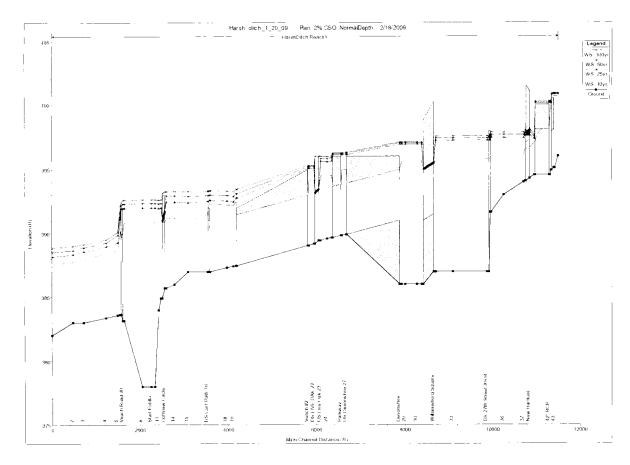


Figure 6 Proposed Improvements Profile (0% CSA) without backwater

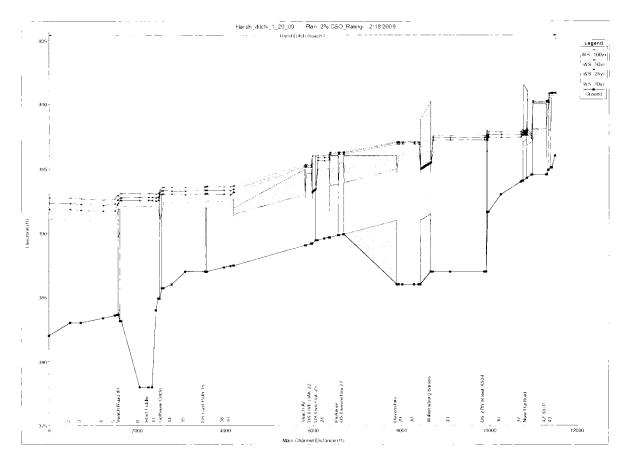


Figure 7 Proposed Improvements Profile (0% CSA) with backwater

Table 10 is a comparison of water surface elevations between Harsh Ditch as it functions today and as it will function with proposed improvements and runoff contribution from additional combination sewer area. The table gives elevations at lettered cross sections determined in the published 1997 FIS for both backwater and without backwater scenarios. It can be seen that the proposed improvements will decrease the water surface elevation throughout Harsh Ditch with the exception of the most downstream cross sections. The increased water surface elevations can be attributed to a more efficient conveyance system which channelizes previously unchannelized runoff that collected in localized depressions and routes it downstream.

The engineer emphasizes that this project is an initial step taken by the City of Owensboro to alleviate flooding and that future improvements to the Harsh Ditch drainage system will be necessary to reduce water surface elevations downstream. The goal of this project as mentioned earlier is to construct a main storm-water conveyance system that can be added to in the future. Future storm water improvement projects planned for Harsh Ditch are additional storm water retention basins and enlargement of portions of the upstream channel and hydraulic structures.

Table 10
Existing Conditions Model Compared To Proposed Improvements Model

Cross Section	Existing Regulatory ¹ 0% CSO Without Improvements (FEET NAVD)	Proposed Regulatory ¹ 2% CSO (FEET NAVD)	Diff. (FEET)	Existing Without Floodway ² 0% CSO Without Improvements (FEET NAVD)	Proposed Without Floodway ² 2% CSO (FEET NAVD)	Diff. (FEET)
Harsh Ditch						
B*	392.1	392.8	+0.7	388.9	389.0	+0.9
C	392.6	393.1	+0.5	392.1	392.6	+0.5
D	392.6	393.1	+0.5	392.1	392.6	+0.5
E	392.9	393.1	+0.2	392.6	393.3	+0.3
F	394.0	393.6	-0.4	393.9	393.3	-0.6
G	394.0	393.6	-0.4	393.9	393.3	-0.6
H	394.0	393.6	-0.4	393.9	393.3	-0.6
l ï	394.0	393.6	-0.4	393.9	393.3	-0.6
j	394.0	393.6	-0.4	393.9	393.3	-0.6
K	394.0	393.6	-0.4	393.9	393.3	-0.6
L	394.8	393.8	-1.0	394.8	393.6	-1.2
М	395.7	395.3	-0.4	395.7	395.3	-0.4
N	396.0	395.3	-0.7	396.0	395.3	-0.7
0	396.4	396.0	-0.4	396.4	396.3	-0.1
Р	398.0	397.1	-0.9	398.0	397.1	-0.9
Q	398.2	397.1	-1.1	398.1	397.1	-1.0
R	398.9	397.6	-1.3	398.3	397.6	-0.7
S	399.4	397.6	-1.8	399.4	397.6	-1.8

¹With Backwater

Conclusion

The proposed drainage improvements for Harsh Ditch will significantly reduce the standing water in the streets and provide additional protection to homes and businesses for storm events at or less than the 25yr event. Although the removal of all standing water in the streets will not be accomplished by this project the depth of water in the streets will be greatly diminished and allow passage of emergency vehicles during the 25yr event. In addition to the reduction of flood elevations this project will allow additional storm-water currently entering the combination sewer system to be redirected to Harsh Ditch, thus decreasing the amount of influent to the wastewater treatment plant.

²Without Backwater

^{*}Locations with water surface increase greater than 0.5ft

ARMY CORPS OF ENGINEERS PERMIT APPLICATION

APPLICATION FOR DEPARTMENT OF THE ARMY PERMIT (33 CFR 325)

OMB APPROVAL NO. 0710-003

Public reporting burden for this collection of information is estimated to average 5 hours per response, including the time for reviewing instructions, Searching existing data sources, gathering and maintaining the data needed, and completing and reviewing the collection of information. Send comments regarding this burden estimate or any other aspect of this collection of information, including suggestions for reducing this burden, to Department of Defense, Washington Headquarters Service Directorate of Information Operations and Reports, 1215 Jefferson Davis Highway, Suite 1204, Arlington VA 22202-4302; and to the Office of Management and Budget, Paperwork Reduction Project (0710-003), Washington, DC 20503. Please DO NOT RETURN your form to either of those addresses. Completed applications must be submitted to the District Engineer having jurisdiction over the location of the proposed activity.

PRIVACY ACT STATEMENT

Authority: 33 USC 401, Section 10; 1413, Section 404. Principal Purpose: These laws require permits authorizing activities in, or affecting, navigable waters of the United States; the discharge of dredged or fill material into waters of the United States, and the transportation of dredged material for the purpose of dumping it into ocean waters. Routine uses: Information provided on this form will be used in evaluating the application for a permit. Disclosure: Disclosure of requested information is voluntary. If information is not provided, however, the permit application cannot be processed nor can a permit be issued.

(ITEMS 1 THRU 4 TO BE FILLED BY THE CORPS)

One set of original drawings or good reproducible copies which show the location and character of the proposed activity must be attached to this application (see sample drawings and instructions) and be submitted to the District Engineer having jurisdiction over the proposed activity. An application that is not completed in full will be returned.

NAME TO STATISTICS OF STATISTI						
APPLI	CANT'S SIGNATURE		DATE			
application and to furnish, upon r	equest, supplemental information i	n support of this permit appli	cation.			
Thereby additionize			agent in the processing of this			
L haraby authoriza R. Di	avid Weaver, PE, PLS	to not in my babalf	acoust in the numerocoins of this			
11.	STATEMENT OF	AUTHORIZATION				
b. Business 270-687-8646		b. Business 270-685-2811				
a. Residence		a. Residence 270-316-7235				
7. APPLICANT'S PHONE NUMBER	RS WITH AREA CODE	10. AGENT'S PHONE NUMBERS WITH AREA CODE				
Owensboro, KY 42302-9003		Owensboro, KY 42301				
P.O. Box 10003		Bryant Engineering, Inc.	/ 4156 Benttree Drive			
6. APPLICANT'S ADDRESS		9. AGENT'S ADDRESS				
City of Owensboro / Joe	Schepers, PE City Eng.	R. David Weaver, Pt	E; Project Manager			
5. APPLICANT'S NAME		8. AUTHORIZED AGENT'S	NAME & TITLE (an agent is not required)			
	(ITEMS BELOW TO BE	FILLED BY APPLICAN	T)			
1. APPLICATION NO.	2. FIELD OFFICE CODE	3. DATE RECEIVED	4. DATE APPLICATION COMPLETED			

NAME, LOCATION, AND DESCR	RIPTION OF PROJECT OR ACTIVITY			
12. PROJECT NAME OR TITLE (see instructions)				
Harsh Ditch, Phase II				
13. NAME OF WATERBODY, IF KNOWN (if applicable)	14. PROJECT STREET ADDRESS (if applicable)			
Harsh Ditch	From the lower Veach Road crossing downstream to the US 2155 crossing located in the City of Owensboro, KY			
15. LOCATION OF PROJECT				
Daviess KY				
COUNTY STATE				
16. OTHER LOCATION DESCRIPTIONS, IF KNOWN (see instruction	ns)			

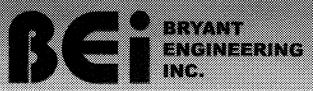
The project follows the existing alignment of Harsh Ditch and parallels Veach Road. In general the project is located in the southeastern quadrant of the City of Owensboro.

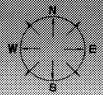
17. DIRECTIONS TO THE SITE

The project can be accessed by taking the north bound exit ramp from the US 60 Bypass onto KY 2155 and traveling north on KY 2155 to the intersection of KY 2155 and E 25th. Turn left onto E 25th Street and left onto Veach Road.

18. NATURE OF ACTIVITY (Description of pro	oject, include all features)				
In general the project will provide drainage improvements to Harsh Ditch from the lower crossing of Veach Road to the KY 2155 crossing. A more detail listing is provided on the attached sheet labeled "#18 NATURE OF ACTIVITY".					
19. PROJECT PURPOSE (Describe the reason	or purpose of the project, see inst.	ructions)			
The project has two primary goals: 1.) To lessen the impacts of flooding in the project area. 2.) To improve the drainage system to allow for future combined sewer seperation projects.					
USE BLOCKS 20-22 IF DREDGED AND/OR FILL MATERIAL IS TO BE DISCHARGED					
20. REASON(S) FOR DISCHARGE					
N/A					
21. TYPE(S) OF MATERIAL BEING DISCHAF	rged and the amount of I	EACH TYPE IN CUBIC YARDS			
22. SURFACE AREA IN ACRES OF WETLAN	IDS OR OTHER WATERS FILLE	ED (see instructions)			
N/A		` <i>'</i>			
23. IS ANY PORTION OF THE WORK ALREA	ADY COMPLETE? YES	NO F YES, DESCR	IBE THE WORK		
24. ADDRESSES OF ADJOINING PROPERTY can be entered here, please attach a supplementa Note attached sheet labeled "#24 ADJO	l list)	HOSE PROPERTY ADJOINS THE WATE	RBODY (If more than		
25. LIST OF OTHER CERTIFICATIONS OR	APPROVALS/DENIALS RECEIVED FOR WORK DESCRIBED IN TH		OR LOCAL AGENCIES		
AGENCY TYPE APPROVAL*	DENTIFICATION NUMBER	DATE APPLIED DATE APPROVE	D DATE DENIED		
KY Division of Water 401 Water Quality		Permit to be submitted			
* Would include but is not restricted to zoning, building and flood plain permits.					
26. Application is hereby made for a permit in this application is complete and accurate. acting as the duly authorized agent of the applications.	I further certify that I possess	rk described in this application. I certif s the authority to undertake the work d	y that the information escribed herein or am		
SIGNATURE OF APPLICANT	DATE	SIGNATURE OF AGENT	DATE		
The application must be signed by the person who desires to undertake the proposed activity (applicant) or it may be signed by a duly authorized agent if the statement in block 11 has been filled out and signed. 18 U.S.C. Section 1001 provides that: Whoever, in any manner within the jurisdiction of any department or agency of the United States knowingly and will fully falsifies, conceals, or covers up any trick, scheme, or disguises a material fact or makes any false, facticious, or fraudulent statements or representations or makes or uses any false writing or document knowing same to contain any false, fictitious or fraudulent statements or entry, shall be fined not more than \$10,000 or imprisoned not more than five years or both.					

ENG FORM 4345 – ONLINE CESPK-CO-R





4215 Bentres Drive P.O. Box 21382 Owenspore, Kentucky 42304

> Office: 270-685-2811 Fax: 270-683-4991

ENGINEERING • LAND SURVEYING

TRANSMITTAL

Date:	02/26/09	_	Project Number:	07-3968
RE:	Harsh Ditch, Phase II		To:	Joel Murphy
	City of Owensboro	_		Kentucky Division of Water
		_		14 Reilly Road
				Frankfort, KY 40601
				Ph: 502-564-6716
			,	
Qty	Description			
11	Stream Encroachment Pen	mit		
		and the second s		
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	***************************************		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	
Remarks	•			
				*
	•			
Signed:	R. David Weaver, PE, PLS			
CC:	Joe Schepers, PE, SE Jim Mischel			
-				